# Optimal Control for Speed Harmonization of Automated Vehicles

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Abstract—This paper addresses the problem of controlling the speed of a number of automated vehicles before they enter a speed reduction zone on a freeway. We formulate the control problem and provide an analytical, closed-form solution that can be implemented in real time. The solution yields the optimal acceleration/deceleration of each vehicle under the hard safety constraint of rear-end collision avoidance. The effectiveness of the solution is evaluated through a microscopic simulation testbed and it is shown that the proposed approach significantly reduces both fuel consumption and travel time. In particular, for three different traffic volume levels, fuel consumption for each vehicle is reduced by 19-22% compared to the baseline scenario, in which human-driven vehicles are considered, by 12-17% compared to the variable speed limit algorithm, and by 18-34% compared to the vehicular-based speed harmonization (SPD-HARM) algorithm. Similarly, travel time is improved by 26-30% compared to the baseline scenario, by 3-19% compared to the VSL algorithm, and by 31-39% compared to the vehicular-based SPD-HARM algorithm.

Index Terms—Vehicle speed control, speed harmonization, automated vehicles, optimal control, energy usage.

## I. INTRODUCTION

# A. Motivation

I N a rapidly urbanizing world, we need to make fundamental transformations in how we use and access transportation. We are currently witnessing an increasing integration of our energy, transportation, and cyber networks, which, coupled with the human interactions, is giving rise to a new level of complexity in transportation [1]. As we move to increasingly complex transportation systems [2], new control approaches are needed to optimize the impact on system behavior of the

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interaction between vehicles at different traffic scenarios. Intersections, merging roadways, speed reduction zones along with the drivers' responses to various disturbances are the primary sources of bottlenecks that contribute to traffic congestion [3]. In 2015, congestion caused people in urban areas in US to spend 6.9 billion hours more on the road and to purchase extra 3.1 billion gallons of fuel, resulting in a total cost estimated at \$160 billion [4].

Speed harmonization (SPD-HARM) is one of the major applications operated in the US towards reducing congestion. SPD-HARM encompasses a group of strategies that are intended to form a series of coherent traffic stream along the roadway by regulating traffic speeds, especially when there is a need of reducing speed down the road. The fundamental idea of such SPD-HARM is to mitigate the loss of highway performance by preventing traffic break-downs and keeping bottlenecks operating at constant traffic feeds. Such idea is well represented in the fundamentals of traffic flow theory: a traffic break-down at the bottleneck can be prevented by progressively guiding the upstream traffic to equal the downstream traffic flow, so the upstream traffic smoothly runs into the downstream traffic and can pass through the bottleneck without disruptions. Using such insight, numerous SPD-HARM strategies were developed with various approaches to determine the speed policy for the system. The control algorithms for SPD-HARM may include traditional intelligent transportation systems technologies or use information provided to connected and automated vehicles (CAVs) to enforce speed on individual vehicle - the individual vehicle-based SPD-HARM can be viewed as an extreme form of VSL. It is important to note that the vehicular-based SPD-HARM should be differentiated from any type of cruise control system such as adaptive cruise control (ACC) or cooperative ACC (CACC), since the latter are intended to facilitate the inter-vehicular interactions by maintaining a given distance between the vehicles as opposed to a strategy for vehicles to approach a bottleneck.

In this paper, we develop a framework that allows each vehicle to optimally control its speed before entering a bottleneck. The objective is to derive the optimal acceleration/deceleration of each vehicle within a "control zone" of appropriate length right before entering a speed reduction zone. The latter causes bottleneck that builds up as vehicles exceed the bottleneck capacity. By optimizing the vehicles' acceleration/deceleration in the upstream, the time of reaching the speed reduction zone is controlled optimally, and thus the recovery time from the congested area is minimized. In addition, the vehicle can

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minimize stop-and-go driving, thereby conserving momentum and energy. Eliminating the vehicles' stop-and-go driving aims at minimizing transient engine operation, and thus we can have direct benefits in fuel consumption [5].

The contributions of this paper are: (1) the problem formulation to control optimally the speed of a number of automated vehicles cruising on a freeway before they enter a speed reduction zone, (2) the analytical, closed-form solution of this problem along with a rigorous analysis that reveals the conditions under which the rear-end collision avoidance constraint does not become active within the control and speed reduction zones, and (3) bringing the gap between the existing algorithms and the real-world implications by providing an optimal solution that can be implemented in a vehicle in real time. We should emphasize that our approach focuses on controlling optimally the vehicle before its entry to the speed reduction zone and not on inter-vehicle interactions, and threfore, it is fundamentally different from the ACC and CACC approaches.

### B. Literature Review

Traditionally, the SPD-HARM has been realized through variable message signs (VMS), variable speed limit (VSL) and the rolling SPD-HARM (a.k.a., pace-car technique) [6]. Both VMS and VSL systems employ the display gantries mounted along roadways to deliver messages or control schemes. Another method for SPD-HARM is the rolling SPD-HARM, which uses designated patrol vehicles entering the traffic to hold a traffic stream at a lower speed, and thus, traverse the congestion area smoothly while mitigating shock waves.

The application of SPD-HARM has been mainly evolved through VSL which appeared to be more effective and efficient than VMS and the rolling SPD-HARM [6], [7]. State-of-theart VSL method employs a proactive approach applying a control action beforehand and then anticipate the behavior of the system (traffic) [8]. Even though this proactive approach has made VSL a popular method over the years, it provides a sub-optimal solution since it is based on a heuristic approach [9].

SPD-HARM methods can be broadly categorized into the (1) reactive approach and (2) proactive approach. The reactive approach initiates the operation at a call upon a queue is detected, and it uses immediate traffic condition information to determine the control strategy for the subsequent time interval. While the reactive approach allows to remedy the bottleneck with real-time feedback operations, it has limitations related to time lag between the occurrence of congestion and applied control [8]. In contrast, the proactive approach has the capability of acting proactively, while anticipating the behavior of traffic flow [8]. Thus, it can predict bottleneck formations before they even occur, while potential shock waves can be resolved by restricting traffic inflow. In addition, the nature of predictions of proactive VSL methods allows for a systematic approach for network-wide coordination which supports system optimization, whereas reactive approach is restrained to a localized control logic.

1) Reactive Speed Harmonization: The first field implementation of SPD-HARM was the VSL system in the German motorway A8 corridor in Munich extended to the boundary of Salzburg, Austria in 1965 [10]. During the early 1960s, US implemented SPD-HARM using VMS on a portion of the New Jersey Turnpike [7]. These SPD-HARM systems required human interventions to determine the messages or speed limits based on the conditions such as weather, traffic congestion and construction schedules. Since 1970s, advances in sensor technologies and traffic control systems allowed the SPD-HARM to automatically operate based on the traffic flow or weather conditions using various types of detectors. The earlier VMS and VSL implementations were often at the purpose of addressing safety issues under work zone areas or inclement weather conditions [7]. In 2007, SPD-HARM started focusing on improving traffic flow mobility. The VSL systems implemented in the M42 motorway at Birmingham, UK, and Washington State Department of Transportation [11], use algorithms which are automatically activated based on predefined threshold of flow and speed measured by detectors embedded in the pavement. The systems display the lowered speed limit within a "control zone" of a pre-defined length [12].

There has been a significant amount of VSL algorithms proposed in the literature to date. A reactive approach for VSL to improve safety and mobility at work-zone areas [13], [14] outperformed the existing VSL algorithms, especially with the traffic demand fluctuations [13]. A simulation-based study show that the performance of VSL is a function of the traffic volume levels [15]. After reaching a particular traffic volume level, the benefit can become more apparent, and therefore, VSL needs to be integrated with ramp metering control [15]. Another VSL algorithm, which implemented in the Twin Cities Metropolitan area, can identify the moving jam based on the deceleration rate between adjacent spots [16]. The field evaluation showed the reduction in average maximum deceleration by 20% over the state-of-the-art in that area while improved the vehicle throughput at the bottleneck areas. Through its evolution, reactive SPD-HARM has consistently showed improvements in many aspects such as reliability, safety and environmental sustainability by providing adequate feedback to the dynamic traffic conditions. However, the capability of reactive control is limited as it can be only effective after a bottleneck occurs while it mainly depends on heuristics.

2) Proactive Speed Harmonization: The necessity of a systematic approach for preventing adverse impacts from impending shock waves eventually led to the development of the proactive VSL. The proactive VSL approach was first proposed in [17] adopting Kalman Filter aimed at estimating impending traffic status based on the time-series traffic measurements [18]. Given the estimated traffic flow, the proactive VSL approach derives a control policy that minimizes various cost functions (e.g., average travel time, summation of square densities of all sections). Although this effort initiated prediction-based VSL systems, the prediction using a time-series approach is not robust, especially under unexpected traffic flow disturbances, since it heavily relies on the empirical patterns.

A pioneering effort in developing a proactive VSL system was made in [19] using model predictive control (MPC). The key aspect of that work is that it prevents traffic breakdown by decreasing the density of approaching traffic rather than focusing on reducing the speed variances. Using MPC, which enabled a network-wide optimization, a series of VSL systems can be coordinated for system-wide optimization that eventually aims at preventing upstream delays. Another MPC-based proactive VSL system was proposed in [20] focusing on creating a discharge section immediate upstream of the bottleneck to regulate traffic flow into the bottleneck that remains close to its capacity. With the intention to influence the motorway mainstream, a traffic flow control approach was proposed in [21], using a suitable feasible-direction algorithm [22], that can yield feedback control policies. These approaches have showed substantial improvements in vehicle throughput, safety, equity, and driver acceptance through microscopic simulation studies [19], [21], [23], [24]. However, there are significant challenges in practical applications associated with computational requirements. Using shock wave theory, another VSL algorithm was proposed in [25] that predicts future traffic evolution based on the different traffic states along the freeway segments. By identifying the location of the front boundaries of shock waves and the active speed limits, the algorithm maximizes the discharge rate at the bottleneck [25].

The performance of SPD-HARM can vary depending on the control approach, characteristics of the topology, and driving behavior. The potential travel time improvements through SPD-HARM have been debatable during peak hours [6], [16]. However, it has been widely agreed that SPD-HARM increases vehicle throughput at the bottleneck. It has been shown that the vehicle throughput can be increased by 4-5% via VSL system [12] and by 5-10 % via rolling SPD-HARM implemented in European countries with significant benefits in safety since personal injury crashes reduced about 30-35% [26]. The environmental impacts of SPD-HARM were also substantial demonstrating reduction in vehicle emissions by 4-10% (depending on the pollutants) [26], and fuel consumption by 4% [12].

Although previous research efforts reported in the literature have aimed at enhancing our understanding of SPD-HARM algorithms, deriving in real time an optimal solution for each individual vehicle under the hard safety constraints still remains a challenging control problem. In this paper, we address the problem of controlling the speed of a number of automated vehicles before they enter a speed reduction zone on a freeway. We formulate the control problem and provide an analytical, closed-form, optimal solution that can be implemented in real time. The solution yields the optimal acceleration/deceleration for each vehicle in the upstream, and thus it controls the time that each vehicle enters the speed reduction zone. Furthermore, we provide the conditions under which the rear-end collision avoidance constraint does not become active at any time within the control and speed reduction zones. By controlling the time of reaching the speed reduction zone, the recovery time from the congested area is reduced which, in turn, leads to the increase in average speed, and eventually, travel time. In addition, vehicles avoid getting into a stop-and-go driving mode, thereby conserving



Fig. 1. Automated vehicles within a control zone approaching a speed reduction zone.

momentum and energy. The unique contribution of this paper hinges on the following three elements: (1) the formulation of the problem of controlling the speed of a number of automated vehicles before they enter a speed reduction zone on a freeway, (2) a rigorous analysis that reveals the conditions under which the rear-end collision avoidance constraint does not become active, and (3) the proposed optimal control framework bridges the gap between the existing algorithms and the real-world implications by providing an optimal solution that can be implemented in a vehicle in real time.

## C. Organization of the Paper

The remaining paper proceeds as follows. In Section II we introduce the model, present the assumptions of our approach and formulate the optimal control problem. In Section III, we provide the control framework, derive an analytical closed-form solution, and discuss the conditions under which the rear-end collision avoidance constraint does not become active. Finally, we provide simulation results and discusion in Section IV and concluding remarks in Section V.

### **II. PROBLEM FORMULATION**

## A. Modeling Framework

We address the problem of optimally controlling the speed of vehicles cruising on a freeway (Fig. 1) that includes a *speed reduction zone* of a length S. The speed reduction zone is a bottleneck that builds up as vehicles exceed the bottleneck capacity. The freeway has a *control zone* right before the speed reduction zone, inside of which the vehicles need to accelerate/decelerate optimally so as to enter the speed reduction zone with the appropriate speed. Therefore, the speed of the potential queue built-up inside the reduction zone is controlled, and thus the congestion recovery time can be optimized. The distance from the entry of the control zone until the entry of the speed reduction zone is L.

We consider a number of automated vehicles  $N(t) \in \mathbb{N}$  in each lane, where  $t \in \mathbb{R}^+$  is the time, entering the control zone (Fig. 1). Let  $\mathcal{N}(t) = \{1, \dots, N(t)\}$ , be the queue in one of the lanes inside the control zone. The dynamics of each vehicle  $i \in \mathcal{N}(t)$  are represented by a state equation

$$\dot{x_i} = f(t, x_i, u_i), \quad x_i(t_i^0) = x_i^0,$$
(1)

where  $t \in \mathbb{R}^+$ ,  $x_i(t)$ ,  $u_i(t)$  are the state of the vehicle and control input,  $t_i^0$  is the time that vehicle *i* enters the control

zone, and  $x_i^0$  is the value of the initial state. For simplicity, we model each vehicle as a double integrator, e.g.,  $\dot{p}_i = v_i(t)$  and  $\dot{v}_i = u_i(t)$ , where  $p_i(t) \in \mathcal{P}_i$ ,  $v_i(t) \in \mathcal{V}_i$ , and  $u_i(t) \in \mathcal{U}_i$  denote the position, speed and acceleration/deceleration (control input) of each vehicle  $i \in \mathcal{N}(t)$  inside the control zone. Let  $x_i(t) = [p_i(t) v_i(t)]^T$  denote the state of each vehicle i, with initial value  $x_i^0 = [0 v_i^0]^T$ , taking values in the state space  $\mathcal{X}_i = \mathcal{P}_i \times \mathcal{V}_i$ . The sets  $\mathcal{P}_i, \mathcal{V}_i$  and  $\mathcal{U}_i, i \in \mathcal{N}(t)$ , are complete and totally bounded subsets of  $\mathbb{R}$ . The state space  $\mathcal{X}_i$  for each vehicle i is closed with respect to the induced topology on  $\mathcal{P}_i \times \mathcal{V}_i$  and thus, it is compact.

We need to ensure that for any initial state  $(t_i^0, x_i^0)$  and every admissible control u(t), the system (1) has a unique solution x(t) on some interval  $[t_i^0, t_i^m]$ , where  $t_i^m$  is the time that vehicle  $i \in \mathcal{N}(t)$  enters the speed reduction zone. To ensure that the control input and vehicle speed are within a given admissible range, the following constraints are imposed:

$$u_{i,min} \leq u_i(t) \leq u_{i,max}, \text{ and}$$
  
 $0 \leq v_{min} \leq v_i(t) \leq v_{max}, \quad \forall t \in [t_i^0, t_i^m], \quad (2)$ 

where  $u_{i,min}$ ,  $u_{i,max}$  are the minimum and maximum control inputs (maximum deceleration/ acceleration) for each vehicle  $i \in \mathcal{N}(t)$ , and  $v_{min}$ ,  $v_{max}$  are the minimum and maximum speed limits respectively. For simplicity, in the rest of the paper we do not consider vehicle diversity, and thus, we set  $u_{i,min} = u_{min}$  and  $u_{i,max} = u_{max}$ .

To ensure the absence of any rear-end collision of two consecutive vehicles traveling on the same lane, the position of the preceding vehicle should be greater than or equal to the position of the following vehicle plus a safe distance  $\delta(t) < S$ , which is a function of speed. Thus, we impose the rear-end safety constraint

$$s_i(t) = p_{i-1}(t) - p_i(t) \ge \delta(v_{ave}(t)), \quad \forall t \in [t_i^0, t_i^m],$$
 (3)

where i - 1 denotes the vehicle which is physically immediately ahead of *i* in the same lane, and  $v_{ave}(t)$  is the average speed of the vehicles inside the control zone at time *t*.

In the modeling framework described above, we impose the following assumptions:

Assumption 1: The vehicles do not change lanes inside the control and speed reduction zones.

Assumption 2: Each vehicle cruises inside the speed reduction zone with the imposed speed limit,  $v_{srz}$ .

Assumption 3: Each vehicle *i* has proximity sensors and can measure local information without errors or delays.

We briefly comment on the above assumptions. The first assumption is considered to simplify the problem and limit the scope of the paper on understanding the implications of the proposed approach on each lane seperately. The second assumption is intended to enhance safety awareness, but it could be modified appropriately, if necessary. The third assumption may be a strong requirement to impose. However, it is relatively straightforward to extend our framework in the case that it is relaxed, as long as the noise in the measurements and/or delays are bounded. For example, we can determine the uncertainties of the state of the vehicle as a result of sensing or communication errors and delays, and incorporate these into the safety constraints.

# B. Optimal Control Problem Formulation

We consider the problem of deriving the optimal acceleration/deceleration of each automated vehicle in a freeway (Fig. 1), under the hard safety constraint to avoid rear-end collision. The potential benefits of the solution of this problem are substantial. By controlling the speed of the vehicles in the upstream or tighten the inflow traffic, the speed of queue builtup decreases, and thus the congestion recovery time is also reduced. Even though the speed of each vehicle is reduced, the throughput at the speed reduction zone is maximized. Moreover, by minimizibg the acceleration/deceleration of each vehicle, we minimize transient engine operation, thus we can have direct benefits in fuel consumption [5] and emissions since internal combustion engines are optimized over steady state operating points (constant torque and speed) [27], [28].

When a vehicle *i* enters the control zone, it receives some information from the vehicle i - 1 which is physically located ahead of it.

Definition 1: For each vehicle *i* when it enters a control zone, we define the *information set*  $Y_i(t)$  as

$$Y_i(t) \triangleq \left\{ p_i(t), v_i(t), t_i^m \right\}, \quad \forall t \in [t_i^0, t_i^m], \tag{4}$$

where  $p_i(t), v_i(t)$  are the position and speed of vehicle *i* inside the control zone, and  $t_i^m$ , is the time targeted for vehicle *i* to enter the speed reduction zone. Each vehicle  $i \in \mathcal{N}(t)$ has proximity sensors and can observe and/or estimate the information in  $Y_i(t)$  without errors or delays (Assumption 3). Note that once vehicle *i* enters the control zone, then immediately all information in  $Y_i(t)$  becomes available to *i*:  $p_i(t), v_i(t)$  are read from its sensors and  $t_i^m$  can also be computed at that time based on the information the vehicle *i* receives from i - 1 as described next.

The time  $t_i^m$  that the vehicle *i* will be entering the speed reduction zone is restricted by the imposing rear-end collision constraint. Therefore, to ensure that (3) is satisfied at  $t_i^m$  we impose the following condition

$$t_{i}^{m} = \max\left\{\min\left\{t_{i-1}^{m} + \frac{\delta(v_{ave}(t))}{v_{i-1}(t_{i-1}^{m})}, \frac{L}{v_{min}}\right\}, \frac{L}{v_{i}(t_{i}^{0})}, \frac{L}{v_{max}}\right\},$$
(5)

where  $v_{i-1}(t_{i-1}^m)$  is the speed of the vehicle i - 1 at the time  $t_{i-1}^m$  that enters the speed reduction zone, and it is equal to the speed,  $v_{srz}$ , imposed inside the reduction zone (Assumption 2). Thus, the condition (5) ensures that the time  $t_i^m$  that vehicle i will be entering the speed reduction zone is feasible and can be attained based on the imposed speed limits inside the control zone. In addition, for low traffic flow where vehicle i - 1 and i might be located far away from each other, there is no compelling reason for vehicle i to accelerate within the control zone. Therefore, in such cases vehicle i can keep cruising within the control zone at  $t_i^0$ . The recursion is initialized when the first

vehicle enters the control zone, i.e., it is assigned i = 1. In this case,  $t_1^m$  can be externally assigned as the desired exit time of this vehicle whose behavior is unconstrained. Thus the time  $t_1^m$  is fixed and available through  $Y_1(t)$ . The second vehicle will access  $Y_1(t)$  to compute the times  $t_2^m$ . The third vehicle will access  $Y_2(t)$  and the communication process will continue with the same fashion until the vehicle N(t) in the queue access the  $Y_{N(t)-1}(t)$ .

We consider the problem of minimizing the control input (acceleration/deceleration) for each vehicle from the time  $t_i^0$  that the vehicle *i* enters the control zone until the time  $t_i^m$  that it enters the speed reduction zone under the hard safety constraint to avoid rear-end collision. By minimizing each vehicle's acceleration/deceleration, we minimize transient engine operation [29], [30], and thus, we can have direct benefits in fuel consumption and emissions since internal combustion engines are optimized over steady state operating points (constant torque and speed).

The optimal control problem is formulated as to minimize the  $L^2$  norm of the control input

$$\min_{u_i} \frac{1}{2} \int_{t_i^0}^{t_i^m} u_i^2(t) dt,$$
  
subject to : (1) and (2), (6)

with initial and final conditions:  $p_i(t_i^0) = 0$ ,  $p_i(t_i^m) = L$ ,  $t_i^0$ ,  $v_i(t_i^0)$ ,  $t_i^m$ , and  $v_i(t_i^m) = v_{srz}$ . Note that we have omitted the rear end safety constraint (3) in the problem formulation above. The analytical solution of the problem including the rear-end collision avoidance constraint may become intractable for real-time implementation. However, in the following section, we provide the conditions under which the rear-end collision avoidance constraint does not become active at any time in  $(t_i^0, t_i^m)$  as long as it is not active at  $t = t_i^0$ . Note that we can guarantee rear-end collision avoidance at time  $t_i^m$  based on (5).

# III. SOLUTION OF THE OPTIMAL CONTROL PROBLEM

For the analytical solution and real-time implementation of the control problem (6), we apply Hamiltonian analysis. In our analysis, we consider that when the vehicles enter the control zone, none of the constraints are active. However, this might not be in general true. For example, a vehicle may enter the control zone with speed higher than the speed limit. In this case, we need to solve an optimal control problem starting from an infeasible state.

The Hamiltonian function is formulated as follows

$$H_{i}(t, x(t), u(t)) = L_{i}(t, x(t), u(t)) + \lambda^{T} \cdot f_{i}(t, x(t), u(t)) + \mu^{T} \cdot g_{i}(t, x(t), u(t)),$$
(7)

where

$$g_i(t, x(t), u(t)) = \begin{cases} u_i(t) - u_{max} \le 0, \\ u_{min} - u_i(t) \le 0, \\ v_i(t) - v_{max} \le 0, \\ v_{min} - v_i(t) \le 0. \end{cases}$$

From (6), the state equations (1), and the control/state constraints (2), for each vehicle  $i \in \mathcal{N}(t)$  the Hamiltonian function becomes

$$H_{i}(t, p(t), v(t), u(t)) = \frac{1}{2}u_{i}^{2} + \lambda_{i}^{p} \cdot v_{i} + \lambda_{i}^{v} \cdot u_{i} + \mu_{i}^{a} \cdot (u_{i} - u_{max}) + \mu_{i}^{b} + (u_{min} - u_{i}) + \mu_{i}^{c} \cdot (v_{i} - v_{max}) + \mu_{i}^{d} \cdot (v_{min} - v_{i}), \qquad (8)$$

where  $\lambda_i^p$  and  $\lambda_i^v$  are the costates, and  $\mu^T$  is a vector of Lagrange multipliers with

$$\mu_i^a = \begin{cases} > 0, & u_i(t) - u_{max} = 0, \\ = 0, & u_i(t) - u_{max} < 0, \end{cases}$$
(9)

$$\mu_i^b = \begin{cases} > 0, & u_{min} - u_i(t) = 0, \\ = 0, & u_{min} - u_i(t) < 0, \end{cases}$$
(10)

$$\mu_i^c = \begin{cases} > 0, \quad v_i(t) - v_{max} = 0, \\ = 0, \quad v_i(t) - v_{max} < 0, \end{cases}$$
(11)

$$\mu_i^d = \begin{cases} > 0, & v_{min} - v_i(t) = 0, \\ = 0, & v_{min} - v_i(t) < 0. \end{cases}$$
(12)

The Euler-Lagrange equations become

$$\dot{\lambda}_i^p = -\frac{\partial H_i}{\partial p_i} = 0, \tag{13}$$

and

$$\dot{\lambda}_{i}^{v} = -\frac{\partial H_{i}}{\partial v_{i}} = \begin{cases} -\lambda_{i}^{p}, & v_{i}(t) - v_{max} < 0 \text{ and} \\ & v_{min} - v_{i}(t) < 0, \\ -\lambda_{i}^{p} - \mu_{i}^{c}, & v_{i}(t) - v_{max} = 0, \\ -\lambda_{i}^{p} + \mu_{i}^{d}, & v_{min} - v_{i}(t) = 0, \end{cases}$$
(14)

with given initial and final conditions  $p_i(t_i^0) = 0$ ,  $p_i(t_i^m) = L$ ,  $v_i(t_i^0)$ , and  $v_i(t_i^m)$ . The necessary condition for optimality is

$$\frac{\partial H_i}{\partial u_i} = u_i + \lambda_i^v + \mu_i^a - \mu_i^b = 0.$$
<sup>(15)</sup>

To address this problem, the constrained and unconstrained arcs need to be pieced together to satisfy the Euler-Lagrange equations and necessary condition of optimality. The analytical solution of (6) without considering state and control constraints was presented in earlier papers [31]–[33] for coordinating in real time CAVs at highway on-ramps and [34] at two adjacent intersections.

When the state and control constraints are not active, we have  $\mu_i^a = \mu_i^b = \mu_i^c = \mu_i^d = 0$ . Applying the necessary condition (15), the optimal control can be given  $u_i + \lambda_i^v = 0$ ,  $i \in \mathcal{N}(t)$ . The Euler-Lagrange equations yield  $\lambda_i^p = -\frac{\partial H_i}{\partial p_i} = 0$  and  $\lambda_i^v = -\frac{\partial H_i}{\partial v_i} = -\lambda_i^p$ . From the former equation we have  $\lambda_i^p = a_i$  and from the latter  $\lambda_i^v = -(a_i t + b_i)$ , where  $a_i$  and  $b_i$  are constants of integration corresponding to each vehicle *i*. Consequently, the optimal control input (acceleration/deceleration) as a function of time is given by

$$u_i^*(t) = a_i t + b_i. (16)$$

Substituting the last equation into the vehicle dynamics equations (1), we can find the optimal speed and position for each vehicle, namely

$$v_i^*(t) = \frac{1}{2}a_i t^2 + b_i t + c_i \tag{17}$$

$$p_i^*(t) = \frac{1}{6}a_i t^3 + \frac{1}{2}b_i t^2 + c_i t + d_i, \qquad (18)$$

where  $c_i$  and  $d_i$  are constants of integration. These constants can be computed by using the initial and final conditions. Since we seek to derive the optimal control (16) in real time, we can designate initial values  $p_i(t_i^0)$  and  $v_i(t_i^0)$ , and initial time,  $t_i^0$ , to be the current values of the states  $p_i(t)$ and  $v_i(t)$  and time t, where  $t_i^0 \le t \le t_i^m$ . Therefore the constants of integration will be functions of time and states, i.e.,  $a_i(t, p_i, v_i), b_i(t, p_i, v_i), c_i(t, p_i, v_i)$ , and  $d_i(t, p_i, v_i)$ . It follows that (17) and (18), along with the initial and terminal conditions, can be used to form a system of four equations of the form  $\mathbf{T}_i \mathbf{b}_i = \mathbf{q}_i$ , namely

$$\begin{bmatrix} \frac{1}{6}t^3 & \frac{1}{2}t^2 & t & 1\\ \frac{1}{2}t^2 & t & 1 & 0\\ \frac{1}{6}(t_i^m)^3 & \frac{1}{2}(t_i^m)^2 & t_i^m & 1\\ \frac{1}{2}(t_i^m)^2 & t_i^m & 1 & 0 \end{bmatrix} \cdot \begin{bmatrix} a_i\\ b_i\\ c_i\\ d_i \end{bmatrix} = \begin{bmatrix} p_i(t)\\ v_i(t)\\ p_i(t_i^m)\\ v_i(t_i^m) \end{bmatrix}, \quad (19)$$

where  $t_i^m$  is specified by (5), and  $v_i(t_i^m) = v_{srz}$  is the imposed speed limit at the speed reduction zone. Hence, we have

$$\mathbf{b}_i(t, p_i(t), v_i(t)) = (\mathbf{T}_i)^{-1} \cdot \mathbf{q}_i(t, p_i(t), v_i(t)), \qquad (20)$$

where  $\mathbf{b}_i(t, p_i(t), v_i(t))$  contains the four integration constants  $a_i(t, p_i, v_i), b_i(t, p_i, v_i), c_i(t, p_i, v_i), d_i(t, p_i, v_i)$ . Thus, (16) can be written as

$$u_i^*(t, p_i(t), v_i(t)) = a_i(t, p_i(t), v_i(t))t + b_i(t, p_i(t), v_i(t)).$$
(21)

Since (19) can be computed in real time, the controller yields the optimal control in real time for each vehicle *i*, with feedback provided through the re-calculation of the vector  $\mathbf{b}_i(t, p_i(t), v_i(t))$  in (20). Similar results can be obtained when the state and control constraints become active [35].

This analytical solution for the case when the state and control constraints are not active, however, does not include the rear-end collision avoidance constraint. Thus, we investigate the conditions under which the rear-end collision avoidance constraint does not become active for any two vehicles i - 1 and i at any time in  $[t_i^0, t_i^m]$ , if they are not active at  $t = t_i^0$ .

Theorem 1: Suppose that there exists a feasible solution of the control problem (6). Then, if  $p_{i-1}(t_i^0) - p_i(t_i^0) = l \ge \delta(v_{ave}(t))$  for i = 2, 3, ..., the constraint (3) does not become active, i.e.,  $p_{i-1}(t) - p_i(t) \ge \delta(v_{ave}(t))$  for all  $t \in (t_i^0, t_i^m]$ , if

$$\left(v_{i-1}(t_i^0) - v_i(t_i^0)\right) \tag{22}$$

$$\geq \frac{\left(l - \delta(v_{ave}(t))\right) \cdot \left(2 \ t^3 - 3 \ t^2 \ t_i^m - (t_i^m)^3\right)}{t_i^m \cdot (t - t_i^m)^2 \cdot t}.$$
 (23)

*Proof:* Without loss of generality and to simplify notation, we reset the time at  $t = t_i^0$ , i.e.,  $t_i^0 = 0$ . Thus, at  $t_i^0 = 0$ , vehicle i - 1 has traveled a distance l inside the control zone and has a speed  $v_{i-1}(t_i^0)$ . Similarly, at  $t_i^0 = 0$ , vehicle i just entered the control zone with an initial speed  $v_i(t_i^0) = v_i^0$ . From (5), the position of vehicle i - 1 will be  $L + \delta(v_{ave}(t))$  at  $t = t_i^m$ 

and will have the speed imposed at the speed reduction zone,  $v_{i-1}(t_i^m) = v_{srz}$ . Similarly, at  $t = t_i^m$  the vehicle *i* will be at the entry of the speed reduction zone, so its position will be *L* and its speed will be the speed imposed at the speed reduction zone,  $v_i(t_i^m) = v_{srz}$ .

Since the state and control constraints are not active, the control input, speed and position for each vehicle *i* are given by (16) - (18). Substituting the conditions at  $t_i^0 = 0$  and  $t = t_i^m$  for the vehicles i - 1 and *i* in (19), and by letting  $\sigma_i = \frac{1}{6}(t_i^m)^3$ ,  $\rho_i = \frac{1}{2}(t_i^m)^2$ , and  $\tau_i = t_i^m$  we have

$$\begin{bmatrix} 0 & 0 & 0 & 1 \\ 0 & 0 & 1 & 0 \\ \sigma_{i} & \rho_{i} & \tau_{i} & 1 \\ \rho_{i} & \tau_{i} & 1 & 0 \end{bmatrix} \cdot \begin{bmatrix} a_{i-1} \\ b_{i-1} \\ c_{i-1} \\ d_{i-1} \end{bmatrix} = \begin{bmatrix} l \\ v_{i-1}(0) \\ L + \delta(v_{ave}(t)) \\ v_{i-1}(t_{i}^{m}) \end{bmatrix}, \quad (24)$$

and

$$\begin{bmatrix} 0 & 0 & 0 & 1\\ 0 & 0 & 1 & 0\\ \sigma_i & \rho_i & \tau_i & 1\\ \rho_i & \tau_i & 1 & 0 \end{bmatrix} \cdot \begin{bmatrix} a_i\\ b_i\\ c_i\\ d_i \end{bmatrix} = \begin{bmatrix} 0\\ v_i^0\\ L\\ v_i(t_i^m) \end{bmatrix}, \quad (25)$$

where  $v_{i-1}(0)$  is the speed of vehicle i-1 at time  $t_i^0 = 0$ ,  $v_{i-1}(t_i^m) = v_i(t_i^m) = v_{srz}$  is the speed at the reduction zone; and  $a_k, b_k, c_k$ , and  $d_k, k = i - 1, i$ , are the constants of integration that can be computed from (20) where

$$(\mathbf{T}_i)^{-1} = \frac{1}{\det(T_i)} \cdot \operatorname{adj}(T), \qquad (26)$$

and det $(T_i) = -\sigma_i \cdot \tau_i + \rho_i^2 = \frac{(t_i^m)^4}{12}$ . Thus from (24) and (25) and by letting  $\gamma_i = \sigma_i - \rho_i \cdot \tau_i$  we

Thus from (24) and (25) and by letting  $\gamma_i = \sigma_i - \rho_i \cdot \tau_i$  we have

$$\begin{bmatrix} a_{i-1} \\ b_{i-1} \\ c_{i-1} \\ d_{i-1} \end{bmatrix} = \frac{1}{\det(T_i)}$$

$$\cdot \begin{bmatrix} \tau_i & -\rho_i + \tau_i^2 - \tau_i & \rho_i \\ -\rho_i & \gamma_i & \rho_i & -\sigma_i \\ 0 & \det(T_i) & 0 & 0 \\ \det(T_i) & 0 & 0 & 0 \end{bmatrix} \cdot \begin{bmatrix} l \\ v_{i-1}(0) \\ L + \delta(v_{ave}(t)) \\ v_{i-1}(t_i^m) \end{bmatrix}$$
(28)

and

$$\begin{bmatrix} a_i \\ b_i \\ c_i \\ d_i \end{bmatrix} = \frac{1}{\det(T_{i-1})}$$
(29)  

$$\cdot \begin{bmatrix} \tau_i & -\rho_i + \tau_i^2 - \tau_i & \rho_i \\ -\rho_i & \gamma_i & \rho_i & -\sigma_i \\ 0 & \det(T_i) & 0 & 0 \\ \det(T_i) & 0 & 0 & 0 \end{bmatrix} \cdot \begin{bmatrix} 0 \\ v_i^0 \\ L \\ v_{i-1}(t_i^m) \end{bmatrix}.$$
(30)

From (28) and (29), we can compute the constants of integration  $a_i, b_i$ 

$$a_{i-1} = \frac{6}{(t_i^m)^2} \cdot \left( v_{i-1}(0) + v_{i-1}(t_i^m) \right)$$
(31)

$$-\frac{12}{(t_i^m)^3} \cdot (L + \delta(v_{ave}(t)) - l),$$
  
$$a_i = \frac{6}{(t_i^m)^2} \cdot (v_i^0 + v_{i-1}(t_i^m)) - \frac{12}{(t_i^m)^3} \cdot L, \qquad (32)$$

$$b_{i-1} = \frac{6}{(t_i^m)^2} \cdot (L + \delta(v_{ave}(t)) - l) - \frac{12}{t_i^m} \cdot \left(\frac{1}{3}v_{i-1}(0) + \frac{1}{6}v_{i-1}(t_i^m)\right),$$
(33)

$$b_{i} = \frac{6}{(t_{i}^{m})^{2}} \cdot L - \frac{12}{t_{i}^{m}} \cdot \left(\frac{1}{3}v_{i}^{0} + \frac{1}{6}v_{i-1}(t_{i}^{m})\right).$$
(34)

The constants of integration  $c_i$  and  $d_i$ , can be derived from (24) and (25) directly

$$c_{i-1} = v_{i-1}(0), \quad c_i = v_i^0, \quad d_{i-1} = l, \text{ and } d_i = 0.$$
 (35)

We know  $p_{i-1}(t_i^0) - p_i(t_i^0) = l \ge \delta(v_{ave}(t))$  for  $i = 2, 3, \ldots$  For all  $t \in (t_i^0, t_i^m]$ , we have

$$p_{i-1}^{*}(t) - p_{i}^{*}(t)$$

$$\geq \delta(v_{ave}(t))$$

$$\Rightarrow \frac{1}{6}(a_{i-1} - a_{i}) \cdot t^{3} + \frac{1}{2}(b_{i-1} - b_{i}) \cdot t^{2} + (c_{i-1} - c_{i}) \cdot t$$

$$+ (d_{i-1} - d_{i}) \geq \delta(v_{ave}(t)), \ t \in (t_{i}^{0}, t_{i}^{m}].$$
(37)

Substituting the constants of integration, (37) becomes

$$\frac{t^{3}}{6} \left[ \frac{6}{(t_{i}^{m})^{2}} \cdot \left( v_{i-1}(0) - v_{i}^{0} \right) + \frac{12}{(t_{i}^{m})^{3}} \cdot \left( \delta(v_{ave}(t)) - l \right) \right] \\ + \frac{t^{2}}{2} \left[ -\frac{4}{t_{i}^{m}} \cdot \left( v_{i-1}(0) - v_{i}^{0} \right) - \frac{6}{(t_{i}^{m})^{2}} \cdot \left( \delta(v_{ave}(t)) - l \right) \right] \\ + t \cdot \left( v_{i-1}(0) - v_{i}^{0} \right) - \left( \delta(v_{ave}(t)) - l \right) \ge 0, \ t \in (t_{i}^{0}, t_{i}^{m}].$$

By rearranging the terms in the last equation we have

$$\left( v_{i-1}(0) - v_i^0 \right) \cdot \left( \frac{t^3}{(t_i^m)^2} - \frac{2t^2}{t_i^m} + t \right) + \left( \delta(v_{ave}(t)) - l \right) \cdot \left( \frac{2t^3}{(t_i^m)^3} - \frac{3t^2}{(t_i^m)^2} - 1 \right) \ge 0, \ t \in (t_i^0, \ t_i^m].$$
(38)

Since  $t_i^m > 0$  we can multiply both sides by  $(t_i^m)^3$ , hence

$$(v_{i-1}(0) - v_i^0) \cdot t \cdot t_i^m \cdot (t - t_i^m)^2 + (\delta(v_{ave}(t)) - l) \cdot (2 \ t^3 - 3 \ t^2 \ t_i^m - (t_i^m)^3) \ge 0,$$
(39)

or

$$\begin{pmatrix} v_{i-1}(t_i^0) - v_i(t_i^0) \end{pmatrix} \\ \geq \frac{\left(l - \delta(v_{ave}(t))\right) \cdot \left(2 \ t^3 - 3 \ t^2 \ t_i^m - (t_i^m)^3\right)}{t_i^m \cdot (t - t_i^m)^2 \cdot t}, \quad t \in (t_i^0, t_i^m].$$

$$(40)$$

 $\frac{Remark \ 1: \ \text{The function}}{\frac{(l-\delta(v_{ave}(t)))\cdot(2 \ t^3-3 \ t^2 \ t_i^m-(t_i^m)^3)}{t_i^m\cdot(t-t_i^m)^2 \ t}, \ t \in (t_i^0, t_i^m], \ \text{in (40) is}$ 

increasing with respect to t for all  $t \in (t_i^0, t_i^m]$ , since  $f'(t) = \frac{(t_i^m)^4 + t^3}{(t-t_i^m)^3} \frac{t^2}{t^2} \ge 0$  for all  $t \in (t_i^0, t_i^s]$ , where  $t_i^s$  is some time such that  $t_i^0 \le t_i^s \le t_i^m$ , and it is decreasing with respect to t for all  $t \in (t_i^s, t_i^m]$ . Therefore, it is sufficient to check the condition (40) at  $t = t_i^s$  where f(t) takes its maximum value.

*Remark 2:* If at time  $t_i^0$  vehicle *i* enters the control zone with an initial speed  $v_i(t_i^0)$  such that there is  $t = t_i^s \in (t_i^0, t_i^m]$  at which the condition (40) does not hold, then vehicle *i* must update  $t_i^m$  given by (5). Let  $(v_{i-1}(t_i^0) - v_i(t_i^0) = \zeta$  and suppose that at  $t = t_i^s \in (t_i^0, t_i^m]$ ,  $\frac{(l-\delta(v_i(t_i^s))) \cdot (2(t_i^{s_3^3-3}(t_i^s)^2 t_i^m - (t_i^m)^3)}{t_i^m \cdot (t_i^s - t_i^m)^2 \cdot t_i^s} > \zeta$ . Then from (40) we have

$$(t_i^m)^3 \cdot (t_i^s \zeta + l - \delta(v_{ave}(t))) - (t_i^m)^2 2 (t_i^s)^2 \zeta + t_i^m \cdot (3 (t_i^s)^2 (l - \delta(v_{ave}(t))) + (t_i^s)^3 \zeta) - 2 (l - \delta(v_{ave}(t))) (t_i^s)^3) \ge 0.$$

$$(41)$$

Therefore, the condition (40) can be satisfied, if vehicle *i* enters the speed reduction zone at time  $\overline{t_i^m}$  which is the solution of (41).

Next, we investigate whether (3) becomes active for any  $t \in (t_i^m, t_{i-1}^f]$ , namely, from the time  $t_i^m$  that vehicle *i* enters the speed reduction zone until the time  $t_{i-1}^f$  vehicle i - 1 exits the speed reduction zone.

Theorem 2: Suppose Assumption 2 is in effect. Then, (3) does not become active, when the state and control constraints are not active, for all  $t \in (t_i^m, t_{i-1}^f]$ , if  $p_{i-1}(t_i^m) - p_i(t_i^m) = l \ge \delta(v_{ave}(t))$ .

*Proof:* Since 
$$v_i(t) = v_{i-1}(t) = v_{srz}$$
, for all  $t \in (t_i^m, t_{i-1}^r]$ ,  
then  $p_{i-1}(t) - p_i(t) = l \ge \delta(v_{ave}(t))$  for all  $t \in (t_i^m, t_{i-1}^f]$ .  $\blacksquare$   
*Corollary 1:* The condition (40) guarantees that (3) does  
not become active for all  $t \in (t_i^0, t_{i-1}^f]$  for the case when the  
state and control constraints are not active.

Remark 3: If  $p_{i-1}(t_i^0) - p_i(t_i^0) = l < \delta(v_{ave}(t))$  for i = 2, 3, ..., then the constraint (3) is active at  $t = t_i^0$ . In this case, the optimal solution would need to start from a non-feasible state. In such instance, the vehicle must be enforced to decelerate as needed until it reaches the minimum safe distance  $\delta(v_{ave}(t))$  from its preceding vehicle before the optimal control applies.

#### **IV. SIMULATION FRAMEWORK AND RESULTS**

To evaluate the effectiveness of our approach, a simulation framework was established as shown in Fig. 2. The analytical, closed-form solution of the proposed control algorithm described in the previous section was implemented for each vehicle using MATLAB Dynamic Link Library (DLL) interface programming to allow data exchange with external programs within the framework. A simulation test-bed network was developed under a microscopic traffic simulation software, VISSIM, and it was integrated into the framework through COM interface.

The mobility measures such as travel time, average speed and vehicle throughput were directly obtained from VISSIM.



Fig. 2. Overview of simulation framework

Fuel consumption is a function of vehicle speed and acceleration. The speed of the vehicle is deemed characteristic of the driver's preference, and thus, it might not be effectively controlled as the driver would override any other than the preferred speed. Moreover, if someone considers to minimize fuel consumption with respect to speed, then the question is how to determine a low bound of the speed since the minimum fuel consumption corresponds to zero vehicle speed. Therefore, through the proposed approach, we consider minimizing the control input (acceleration/deceleration though the gas/brake pedal position) that results in minimizing transient engine operation. If we minimize transient engine operation, we have direct benefits in fuel consumption [5] and emissions since internal combustion engines are optimized over steady state operating points (constant torque and speed) [27], [28]. Fuel consumption was quantified by using the polynomial metamodel proposed in [36] which yields vehicle fuel consumption as a function of speed, v(t), and control input, u(t), namely

$$\dot{f}_v = \dot{f}_{cruise} + \dot{f}_{accel}, \qquad (42)$$

where  $t \in \mathbb{R}^+$  is the time,  $\dot{f}_{cruise} = w_0 + w_1 \cdot v(t) + w_2 \cdot v^2(t) + w_3 \cdot v^3(t)$  estimates the fuel consumed by a vehicle traveling at a constant speed v(t), and  $\dot{f}_{accel} = u(t) \cdot (n_0 + n_1 \cdot v(t) + n_2 \cdot v(t)^2)$  is the additional fuel consumption caused by acceleration u(t). The polynomial coefficients  $w_n$ ,  $n = 0, \ldots, 3$  and  $n_s$ , s = 0, 1, 2 were calculated from experimental data. For the case studies we considered in this paper, all vehicles were the same with the parameters reported in [36], where the vehicle mass was  $M_v = 1, 200 kg$ , the drag coefficient was  $C_D = 0.32$ , the air density was  $\rho_a = 1.184 km/m^2$ , the frontal area was  $A_F = 2.5 m^2$ , and the rolling resistance coefficient was  $\mu = 0.015$ .

## A. Testbed network

The testbed network consists of a single-lane corridor 2,000 m length long (Fig. 3) that includes a speed reduction zone, 300 m length long, located downstream. As the capacity at the entrance of the speed reduction zone drops, the stopand-go congestion is meant to generate under the baseline scenario even without feeding additional volumes through ramps. The speed limit inside the speed reduction zone is set



Fig. 3. Testbed network developed in VISSIM

to be 15.6 m/s. We also designated a 300 m long control zone, right before the speed reduction zone. The length of the control zone was determined appropriately to provide adequate distance to address traffic congestion that would have occurred at the speed reduction zone (i.e., bottleneck) downstream. It is noted that neither the speed limit at the speed reduction zone, nor the length of the control zone affect the analytical, closed-form solution for system optimality. However, by varying the length of the control zone, or the speed limits, might yield different quantitative results on fuel consumption, travel time, and throughput.

It is noted that the VISSIM model was calibrated with a reference to the guideline of the Highway Capacity Manual 2010 [37]. According to this manual, the capacity of two-lane highways under the baseline scenario is indicated as 1,700 veh/h. In this study, however, the design capacity was relaxed to 1,800 veh/h considering that the capacity increases in one-way versus a two-way highway as it has been shown based on empirical data [38], [39]. The minimum safe distance  $\delta(t)$  in VISSIM which is defined as the distance a driver would maintain while following vehicle can be expressed as follows [40]

$$\delta(t) = c_0 + c_1 \cdot v_{ave}(t) \tag{43}$$

where  $c_0$  is the standstill distance between two vehicles,  $c_1$  is the headway time, and  $v_{ave}(t)$  is the average speed. In our case, we used the default value of VISSIM for  $c_0$ , e.g., 1.5 *m*, while  $c_1$  was adjusted to 1.2 *sec*, thereby the maximum traffic flow was approximated at near the desired value of 1, 800 *veh/h*. In addition, some of the key parameters affecting VISSIM's car following behavior used include: the standstill distance of 1.5 *m*, headway time of 1.2 *sec*, and the absolute space difference between minimum and maximum gap distance as 4 *m*.

### B. Experimental Set-Up

To evaluate the effectiveness of the efficiency of the proposed approach under varying traffic volume conditions, we considered the following cases: (i) traffic volume of 1,620 veh/h which is 10% less than the capacity, (ii) traffic volume of 1,800 veh/h at the capacity, and (iii) traffic volume of 1,980 veh/h which is 10% more than the capacity. For all these cases, the total simulation time was 1,000 sec.

TABLE I Control Parameters

Parameter	Value
Min. speed	10 m/s
Max. speed	35m/s
Max. acceleration	$4.5m/s^{2}$
Max deceleration	$-4.5m/s^2$

Five replications of each simulation case were conducted to account for the effect of stochastic components of traffic and drivers' behaviors. It is noted that five replications produced statistically significant results at a 95% confidence level.

The parameters used for the proposed control algorithm are summarized in Table I. According to a guideline published by the Federal Highway Administration [41], suggested maximum acceleration and deceleration are  $3.04 \ m/s^2$  and  $-4.5 \ m/s^2$ , respectively. Several recent studies have employed some relaxed values considering the advancement of the vehicle technologies, in which the thresholds ranged from  $4.0 \ m/s^2$ to  $8.0 \ m/s^2$  [42], [43]. Since the vehicle considered here are automated, the maximum acceleration threshold was assigned to be  $4.5 \ m/s^2$  and the maximum deceleration was adopted as the guideline taking account the safety and comfortable driving behavior. However, these values could be modified as necessary without any implications in the proposed framework.

We considered the following three cases for comparison: (i) a baseline scenario associated with human drivers based on a car-following model, (ii) the state-of-the-art VSL algorithm, SPEed ControllIng ALgorIthm using Shock wave Theory (SPECIALIST) and (iii) the vehicular-based SPD-HARM algorithm proposed by the US DOT [44].

The baseline scenario was designed to emulate the human drivers' behavior using the Wiedemann model in VISSIM [45]. The comparison analysis with the baseline represents the net benefit of the control algorithm over the status that would have been resulted without the control algorithm. The SPECIALIST is a proactive VSL algorithm that projects traffic conditions for a short-term horizon using MPC. It utilizes shock wave theory to generate control speed and duration, and thus, it does not require complicated computation. In addition, it includes only a few parameters with physical interpretations for feasible field implementations. In this study, SPECIALIST was modeled using C# programming and implemented in the VISSIM using its COM interface. Since SPECIALIST is based on a mesoscopic model that utilizes the spot-based measurement collected at a fixed location and aggregated for a certain period of time, detector stations were evenly embedded at every 75 m along the corridor to estimate the local traffic states. The traffic state of each detector station was estimated every 60 sec by using the aggregated estimation of the latest 60 sec interval, and the activation of VSL was examined every 60 sec as well. The vehicles within the control zone were ensured to follow the VSL control speed at 100% compliance rate without perception-reaction time. Such ideal condition was necessary for a fair comparison with the

proposed control algorithm which assumed 100% automated vehicle penetration. The SPECIALIST algorithm had several parameters that can be selected by the operator. For the best performance of the algorithm, the parameters were tuned with several iterations. The thresholds of maximum speed and capacity were chosen as 15.6 m/s and 1,500 veh/h, respectively, which were determined after empirical trials to find the minimum values where traffic congestion was not observed under the VSL implemented at the 100% automated vehicle market penetration.

A vehicular-based SPD-HARM algorithm, called Simple Speed Harmonization (SH) Algorithm [44], was developed through the support of US DOT in an effort to realize field implementation of the SPD-HARM algorithm using CAVs. The simple SPD-HARM algorithm was created in inspiration from the work in [46]; however, it was simplified to demonstrate the feasibility under the context of probe-based traffic management using CAVs, namely

$$s_i(x,t) = m(t) \cdot x_i(t) + b(t) \tag{44}$$

$$m(t) = \frac{s_m(t) - s_n(t)}{\Delta x_{mn}} \tag{45}$$

$$b(t) = s_m(t) \tag{46}$$

where  $s_i(x, t)$  denotes the speed of vehicle *i* at time *t* and  $x_i$  denotes the location of vehicle *i* at time *t*. With successful validation of field implementation [44], the effectiveness of the Simple SH algorithm in mitigating the traffic oscillations has been updated by Learn [47]. The recent results showed that the probe-based SPD-HARM algorithm effectively mitigated the traffic oscillations compared to the base case of no-control, while the travel time and fuel consumption were increased over the base case [47].

## C. Results and Analysis

We investigated acceleration and speed profiles of the four cases including the proposed control algorithm, VSL, vehicle-based SPD-HARM algorithm and human driven vehicles. Figure 4 shows the accelerations and speeds of two adjacent vehicles traveling within the control zone. These trajectories were selected from the simulation testbed at the same time for all cases.

While the speed trajectories of all cases show vehicles enter into the control zone at 31 m/s and exits the control zone around 15.6 m/s, the acceleration profiles are very different. As expected, human drivers case and the variable speed limit (VSL) case showed large fluctuations. The individual speed harmonization (SPD-HARM) case showed better acceleration profile without explicitly considering optimal control within the control zone. Finally, the proposed optimal control case demonstrates the most smooth acceleration profile, which is the linear function as designated by (16).

The results corresponding to fuel consumption, travel time, and throughput of the three approaches (i.e., VSL, vehicular-based SPD-HARM algorithm, and the proposed control algorithm) are shown in Fig. 5 and compared to a baseline scenario using human-driven vehicles. Fuel consumption per



Fig. 4. Accelerations and speeds profiles of all cases



Fig. 5. Comparisons of the baseline scenario (human-driven vehicles), VSL, and optimal control algorithm.

vehicle for the three approaches demonstrate similar patterns with the results of travel time as shown in Fig. 5 (a) and (b). By optimizing the vehicles' acceleration/deceleration inside the control zone, the time of reaching the speed reduction zone is controlled optimally, and thus the recovery time from the congested area is minimized, and therefore we have improvement in travel time as shown in Fig. 5 (b). Furthermore, each vehicle avoids getting into a stop-and-go driving mode, thereby conserving momentum and energy. Eliminating the vehicles' stop-and-go driving aims at minimizing transient engine operation, and thus we have direct benefits in fuel consumption, as shown in Fig. 5 (a), since internal combustion engines are optimized over steady state operating points (constant torque and speed) [5].

The proposed control algorithm significantly reduces fuel consumption of each vehicle by 19-22% over the baseline scenario, by 12-17% over the VSL algorithm, and by 18-34% over the vehicular-based SPD-HARM algorithm for the three traffic volume cases considered here. Using the vehicular-based SPD-HARM, fuel consumption was statistically not different from that of the baseline scenario for the traffic volume cases corresponding to the road capacity and the one of 10% less than capacity. When the traffic volume was 10% more than the capacity, fuel consumption increased by 18% over the baseline scenario. These observations are consistent with the results in an experiment conducted by a research group of FHWA [47].

As shown in Fig. 5 -(b) and (c), the proposed control algorithm improved travel time and throughput for all traffic volumes over the baseline scenario, VSL, and the vehicular-based SPD-HARM algorithm. In particular, travel time was improved by 26-30% over the baseline scenario, by 3-19% over the VSL algorithm, and by 31-39% over the vehicular-based SPD-HARM algorithm for the three traffic volume cases. Both VSL and the proposed control algorithm reduced the travel time and improved the vehicle throughput under all three traffic volume cases. The proposed control algorithm reduced travel time and throughput by 19% and 3% respectively, compare to VSL under the traffic volume higher than the capacity. On the contrary, the vehicular-based SPD-HARM increased travel compared to the baseline scenario. This is a reasonable outcome since the vehicular-based SPD-HARM is designed to lead harmonizing speed throughout the traffic stream, but not optimizing its performance. However, the vehicle throughput at the bottleneck area was not significantly different than those of baseline scenario when the traffic volume was less than the capacity or at the capacity at 95% confidence level as shown in Fig. 5-(c). This implies that even though the vehicular-based SPD-HARM might increase the average travel time along the control zone, it eventually resulted in resolving potential shockwaves at the downstream bottleneck area and helped release the vehicles effectively unless the traffic congestion is initiated.

To ensure statistical significance of the results, the t-tests were conducted for all three measurements by paring up of two scenarios out of the four (i.e., human drivers, VSL, vehicular-based SPD-HARM, and the optimal control algorithm). The statistical results showed that the p-values were close to zeros (rounded at the second decimal points), which implied that the measurement of certain scenario was significantly different from the other two scenarios at the 99th percentile confidence level. The simulation results were further assessed by identifying the minimum sample size using the Sample Size Determination Tool (Version 2.0) which was developed based on the FHWA sample size determination methodology [48]. In the tool, the minimum sample size is determined as a function of the statistics of the initial simulation runs, confidence level and the tolerance error. In this paper, the 95th percentile confidence interval and 5% error tolerance value were used as suggested in the manual [48]. The initial five runs of simulation satisfy the minimum sample

size for all scenarios per the tolerance level of 5% with the 95th percentile confidence level.

It is important to highlight that the proposed control algorithm does not require a phase to clear congestion or a technique to prevent the bottleneck formation in order to improve vehicle throughput. Instead, the vehicle throughput was improved by having all individual vehicles proactively determines their optimal trajectories to the target location while the minimum spacing from the preceding vehicle is assured.

# V. CONCLUDING REMARKS AND FUTURE RESEARCH

In the "new world" of massive amounts of information from vehicles and infrastructure, what we used to model as uncertainty (noise or disturbance) becomes additional input or extra state information in a much higher-dimensional vector. The processing of such multiscale information requires new approaches in order to overcome the curse of dimensionality. Then the question becomes "how much information do we need and what we can do with it?" It seems clear that the availability of this information has the potential to ease congestion, reduce energy usage, and diminish traffic accidents by enabling vehicles to rapidly account for changes in their mutual environment. The approach we proposed in this paper demonstrated that automation and control can be used to improve traffic flow.

In particular, we addressed the problem of optimally controlling the speed of a number of automated vehicles before they enter a speed reduction zone on a freeway. We formulated the control problem and used Hamiltonian analysis to provide an analytical, closed-form solution that can be implemented in real time. The solution yields the optimal acceleration/deceleration of each vehicle under the hard safety constraint of rear-end collision avoidance. The effectiveness of the proposed solution was demonstrated through simulation and it was shown that the proposed approach can reduce both fuel consumption and travel time.

In our proposed framework, we considered that all vehicles are automated. Also we did not consider lane changing. An important direction for future research is to relax these assumptions and investigate the implications in the solution. The assumption of perfect information might also impose barriers in a potential implementation and deployment of the proposed framework. Although it is relatively straightforward to extend our results in the case that this assumption is relaxed, future research should investigate the implications of having information with errors and/or delays to the system behavior. Finally, considering lane changing and mixed traffic (e.g., automated vehicles and human-driven vehicles) would eventually aim at addressing the remaining practical consequences of implementing this framework.

#### References

- A. A. Malikopoulos, "Centralized stochastic optimal control of complex systems," in *Proc. Eur. Control Conf.*, Jul. 2015, pp. 721–726.
- [2] A. A. Malikopoulos, "A duality framework for stochastic optimal control of complex systems," *IEEE Trans. Autom. Control*, vol. 61, no. 10, pp. 2756–2765, Oct. 2016.

- [3] J. Rios-Torres and A. A. Malikopoulos, "A survey on the coordination of connected and automated vehicles at intersections and merging at highway on-ramps," *IEEE Trans. Intell. Transp. Syst.*, vol. 18, no. 5, pp. 1066–1077, May 2017.
- [4] B. Schrank, B. Eisele, T. Lomax, and J. Bak, "2015 urban mobility scorecard," Texas A&M Transp. Inst., Tech. Rep., 2015.
- [5] A. A. Malikopoulos, D. N. Assanis, and P. Y. Papalambros, "Optimal engine calibration for individual driving styles," SAE Tech. Paper 2008-01-1367, 2008.
- [6] M. Roberts, I-70 Pace Cars Experiment is Working, CDOT Says—Just Not All the Time, Jan. 2012. [Online]. Available: http://www.westword.com/news/i-70-pace-cars-experiment-is-workingcdot-says-just-not-all-the-time-5880518
- [7] M. Robinson, "Examples of variable speed limit applications. Speed management workshop," Transp. Res. Board, Tech. Rep., Jan. 2000.
- [8] B. Khondaker and L. Kattan, "Variable speed limit: A microscopic analysis in a connected vehicle environment," *Transp. Res. C, Emerg. Technol.*, vol. 58, pp. 146–159, Sep. 2015.
- [9] J. R. D. Frejo and E. F. Camacho, "Global versus local MPC algorithms in freeway traffic control with ramp metering and variable speed limits," *IEEE Trans. Intell. Transp. Syst.*, vol. 13, no. 4, pp. 1556–1565, Dec. 2012.
- [10] P. Schick, "Influence of traffic control systems on freeway capacity and stability of traffic flow," Univ. Stuttgart, Stuttgart, Germany, Tech. Rep., 2003, p. 20.
- [11] P. Brinckerhoff, T. Farradyne, and J. C. Burgess, "Active traffic management concept of operations," Washington State Dept. Transp., Olympia, WA, USA, Tech. Rep., 2008.
- [12] Mott MacDonald Ltd., "ATM monitoring and evaluation 4-lane variable mandatory speed limits 12 month report," Highways Agency, Bristol, U.K., Tech. Rep., 2008.
- [13] B. Park and S. S. Yadlapati, "Development and testing of variable speed limit control logics for work zones using simulation," Ph.D. dissertation, Washington, DC, USA, 2003.
- [14] P.-W. Lin, K.-P. Kang, and G.-L. Chang, "Exploring the effectiveness of variable speed limit controls on highway work-zone operations," *J. Intell. Transp. Syst.*, vol. 8, no. 3, pp. 155–168, 2004.
- [15] J. Zhicai, Z. Xiaoxiong, and Y. Hongwei, "Simulation research and implemented effect analysis of variable speed limits on freeway," in *Proc. 7th Int. IEEE Conf. Intell. Transp. Syst.*, Oct. 2004, pp. 894–898.
- [16] E. Kwon, C. Park, D. Lau, and B. Kary, "Minnesota variable speed limit system: Adaptive mitigation of shock waves for safety and efficiency of traffic flows," Ph.D. dissertation, Washington, DC, USA, 2011.
- [17] A. Alessandri, A. Di Febbraro, A. Ferrara, and E. Punta, "Optimal control of freeways via speed signalling and ramp metering," *Control Eng. Pract.*, vol. 6, no. 6, pp. 771–780, 1998.
- [18] G. Welch and G. Bishop, "An introduction to Kalman filter," Dept. Comput. Sci., Univ. North Carolina, Chapel Hill, NC, USA, Tech. Rep., 2001.
- [19] A. Hegyi, B. De Schutter, and J. Hellendoorn, "Optimal coordination of variable speed limits to suppress shock waves," *IEEE Trans. Intell. Transp. Syst.*, vol. 6, no. 1, 2005.
- [20] X.-Y. Lu, P. Varaiya, R. Horowitz, D. Su, and S. E. Shladover, "A new approach for combined freeway variable speed limits and coordinated ramp metering," in *Proc. 13th Int. IEEE Conf. Intell. Transp. Syst. (ITSC)*, Sep. 2010, pp. 491–498.
- [21] R. C. Carlson, I. P. M. Papamichail, and A. Messmer, "Optimal mainstream traffic flow control of large-scale motorway networks," *Transp. Res. C, Emerg. Technol.*, vol. 18, no. 2, pp. 193–212, 2010.
- [22] M. Papageorgiou and M. Marinaki, "A feasible direction algorithm for the numerical solution of optimal control problems," Dyn. Syst. Simul. Lab., Tech. Univ. Crete, Chania, Greece, Tech. Rep., 1995.
- [23] A. Hegyi, B. De Schutter, and J. Hellendoorn, "Model predictive control for optimal coordination of ramp metering and variable speed limits," *Transp. Res. C, Emerg. Technol.*, vol. 13, no. 3, pp. 185–209, Jun. 2005.
- [24] A. Hegyi, B. De Schutter, and J. Hellendoorn, "Optimal coordination of variable speed limits to suppress shock waves," *IEEE Trans. Intell. Transp. Syst.*, vol. 6, no. 1, pp. 102–112, Mar. 2005.
- [25] A. Hegyi, S. P. Hoogendoorn, M. Schreuder, H. Stoelhorst, and F. Viti, "SPECIALIST: A dynamic speed limit control algorithm based on shock wave theory," in *Proc. 11th Int. IEEE Conf. Intell. Transp. Syst. (ITSC)*, Oct. 2008, pp. 827–832.

- [26] C. Fuhs, "Synthesis of active traffic management experience in euripe and the United States," U.S. Dept. Transp., Federal Highway Admin., Washington, DC, USA, Tech. Rep. FHWA-HOP-10-031, 2010.
- [27] A. A. Malikopoulos, P. Y. Papalambros, and D. N. Assanis, "Online identification and stochastic control for autonomous internal combustion engines," *J. Dyn. Syst., Meas., Control*, vol. 132, no. 2, p. 024504, 2010.
- [28] A. A. Malikopoulos, "A multiobjective optimization framework for online stochastic optimal control in hybrid electric vehicles," *IEEE Trans. Control Syst. Technol.*, vol. 24, no. 2, pp. 440–450, Mar. 2016.
- [29] M. L. Shaltout, A. A. Malikopoulos, S. Pannala, and D. Chen, "A consumer-oriented control framework for performance analysis in hybrid electric vehicles," *IEEE Trans. Control Syst. Technol.*, vol. 23, no. 4, pp. 1451–1464, Jul. 2015.
- [30] A. A. Malikopoulos, "Stochastic optimal control for series hybrid electric vehicles," in *Proc. Amer. Control Conf. (ACC)*, Jun. 2013, pp. 1189–1194.
- [31] J. Rios-Torres, A. A. Malikopoulos, and P. Pisu, "Online optimal control of connected vehicles for efficient traffic flow at merging roads," in *Proc. IEEE 18th Int. Conf. Intell. Transp. Syst.*, Sep. 2015, pp. 2432–2437.
- [32] J. Rios-Torres and A. A. Malikopoulos, "Automated and cooperative vehicle merging at highway on-ramps," *IEEE Trans. Intell. Transp. Syst.*, vol. 18, no. 4, pp. 780–789, Apr. 2017.
- [33] I. A. Ntousakis, I. K. Nikolos, and M. Papageorgiou, "Optimal vehicle trajectory planning in the context of cooperative merging on highways," *Transp. Res. C, Emerg. Technol.*, vol. 71, pp. 464–488, Oct. 2016.
- [34] Y. J. Zhang, A. A. Malikopoulos, and C. G. Cassandras, "Optimal control and coordination of connected and automated vehicles at urban traffic intersections," in *Proc. Amer. Control Conf.*, Jul. 2016, pp. 6227–6232.
- [35] A. A. Malikopoulos, C. G. Cassandras, and Y. Zhang, "A decentralized energy-optimal control framework for connected automated vehicles at signal-free intersections," *Automatica*, vol. 93, pp. 244–256, Jul. 2018.
- [36] M. A. S. Kamal, M. Mukai, J. Murata, and T. Kawabe, "Ecological vehicle control on roads with up-down slopes," *IEEE Trans. Intell. Transp. Syst.*, vol. 12, no. 3, pp. 783–794, Sep. 2011.
- [37] P. Ryus, M. Vandehey, L. Elefteriadou, R. G. Dowling, and B. K. Ostrom, *Highway Capacity Manual*, 5th ed. Washington, DC, USA: Transportation Research Board., 2010. [Online]. Available: http://www.trb.org/Publications/Blurbs/164718.aspx
- [38] "Discussion of potential one-way street conversions in downtown Ottawa," in Proc. Annu. Meeting Summit, ACT Canada Sustain. Mobility Netw., 2012. [Online]. Available: https://www.actcanada.com/ docs/default-source/act-canada-summit-2013/one-way-streetconversion-discussion-paper.pdf?sfvrsn=2
- [39] C. Spahr. (Feb. 2012). One-Way or Two-Way Streets More Efficient—It Depends on What You Measure. [Online]. Available: http://www.ssti.us/2013/02/one-way-or-two-way-streets-more-efficientit-depends-on-what-you-measure/
- [40] PTV Vision—User Manual. (2014) PTV VISSIM Traffic Simulation User Manual: Version 7. [Online]. Available: http://visiontraffic. ptvgroup.com/en-us/products/ptv-vissim/
- [41] R. Dowling, A. Skabardonis, and V. Alexiadis, "Traffic analysis toolbox: Guidelines for applying traffic microsimulation modeling software," Tech. Rep., 2004, vol. 3.
- [42] J. Lee, B. Park, K. Malakorn, and J. So, "Sustainability assessments of cooperative vehicle intersection control at an urban corridor," *Transp. Res. C, Emerg. Technol.*, vol. 32, pp. 193–206, Jul. 2013.
- [43] H. Zhou, R. Saigal, F. Dion, and L. Yang, "Vehicle platoon control in high-latency wireless communications environment: Model predictive control method," *Transp. Res. Rec., J. Transp. Res. Board*, vol. 2324, pp. 81–90, 2012. [Online]. Available: https://trrjournalonline.trb.org/doi/citedby/10.3141/2324-10
- [44] J. Ma, X. Li, F. Zhou, J. Hu, and B. B. Park, "Parsimonious shooting heuristic for trajectory design of connected automated traffic part II: Computational issues and optimization," *Transp. Res. B, Methodol.*, vol. 95, pp. 421–441, Jan. 2017.
- [45] R. Wiedemann, Simulation des Strassenverkehrsflusses. Schriftenreihe des Instituts f
  ür Verkehrswesen der Universität Karlsruhe, 1974.
- [46] X.-Y. Lu, J. Lee, D. Chen, J. Bared, D. Dailey, and S. E. Shladover, "Freeway micro-simulation calibration: Case study using aimsun and VISSIM with detailed field data," in *Proc. 93rd Annu. Meeting Transp. Res. Board*, 2014. [Online]. Available: https://trid.trb.org/view/1289628
- [47] S. Learn, "Speed harmonization using connected and automated vehicles," Ph.D. dissertation, Univ. Virginia, Charlottesville, VA, USA, 2016.
- [48] Vdot Governance Document Traffic Operations and Safety Analysis Manual (tosam)—Version 2.0, VDOT, Richmond, VA, USA, 2013.



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